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SCOPE OF BAMBOO REINFORCEMENT IN CONCRETE BEAMS FOR LOW-COST HOUSING

F. Falade and E. Ikponmwenso

Department of Civil & Environmental Engineering
University of Lagos, Akoka

Lagos, Nigeria.

E-mail: ffalade@shomail.com & efe_ewacen@yahoo.com

Abstract

This paper investigates the extent to which bamboo reinforcement can be used in concrete beams for low-cost housing. Beams of uniform cross sectional area (225x450mm) were designed to BS 8110. They were simply supported over varying effective spans of 3.0, 3.5, 4.0 and 4.5 metres. The results of the laboratory tests showed that the average 28-day characteristic strength of concrete was 25N/mm² while an average tensile strength of 133.50N/mm² was obtained for bamboo splints of size 10x10mm (square section). The design was based on two limit states: ultimate and serviceability (deflection). The beams were subjected to 20%, 40%, 60%, 80% and 100% of the ultimate moment of resistance of the section ($M_u = 0.156bd^2f_{cu}$). The corresponding uniformly distributed load was computed from the well-known relationship between maximum moment and uniformly distributed load (udl) on beam and its effective span. The udl was imposed on each beam at different percentages of ultimate moment of resistance.

All the beams were singly reinforced. The lever arm curve that permits the area of reinforcement to be calculated for singly reinforced beams was used. The corresponding deflection at each level of loading was calculated for each span. The well established equations of deflection were used with some modifications that reflect the characteristics of the bamboo splints.

The shear stress at each level of loading was calculated and compared with both allowable and ultimate values. The computed deflection for each loading case showed that the deflection of bamboo-reinforced beams increased with increase in value of applied moment and span but reduced with increase in the quantity of bamboo reinforcement in the beams.

The results show that the optimum span for bamboo reinforcement in reinforced concrete beams based on cross-sectional dimensions of 225x450mm is 4000mm at optimum load of 60% M_u . Within this limit, the requirements of ultimate and serviceability limit states were achieved.

1.0 INTRODUCTION

In Nigeria, the problem of low-cost housing cannot be overemphasized. Many attempts have been made both by

the government and private sector to alleviate the problem by providing affordable housing scheme to all categories of the population but such efforts have not been too successful.

Usually a housing project initially conceives low-cost end up being high end housing scheme because of high cost of construction materials. Concrete comprises cement, sand and crushed rock aggregate or gravel combined together in a definite proportion with water depending on the required strength. Concrete structures are often subjected to tensile stresses, hence, their elements are often reinforced against the stresses. The reinforcement is provided in the form of steel, synthetic and glass fibres. The use of bamboo as tensile reinforcement for beams has been extensively studied and found to be adequate for some structural applications. However the strength characteristics vary from species to species and locality of growth. It was reported by Mat Chue (1966) that there were 1350 species of bamboo worldwide. In Nigeria, over the years, the use of steel reinforcement increased progressively to the extent that many people now prefer bungalows (where the use of reinforcement is minimal), to buildings with multiple floors. Inspired by Akeju and Bolade (1997) revealed that one of the species of bamboo (*Bambusa Vulgaris*) in Nigeria could be used as substitute for steel reinforced structural elements. The choice of bamboo is attributed to its reasonably high tensile strength, good availability, its aphoristic nature, its negligible cost and its non-susceptibility to corrosion being a non-metallic reinforcement. The principal problems associated with the use of bamboo as reinforcement were: Volume changes, moisture susceptibility, poor bond and decayancy. Bamboo reinforcement absorbs moisture from fresh concrete and swells, causing severe cracking of concrete before any load is applied. Subsequently, as moisture content decreases, the bamboo shrinks causing separation of bamboo from the matrix.

Different percentages of bamboo cement have been recommended for use in rectangular beams. Cox and Geymayer (1970) recommended an optimum reinforcement

ratio of 3–4% while Bolade and Akeju (2000) recommended an optimum bamboo content of up to 3%. The difference in these results may be attributed either to the bamboo species that was investigated. The low strength of seasoned bamboo indicates its suitability as reinforcement; however, its low modulus of elasticity and its flexibility will require an area of bamboo reinforcement for flexural members of about ten times that of corresponding steel reinforced member.

Kankam et al. (1986) performed laboratory research on simply supported one-way bamboo reinforced concrete slabs subjected to concentrated line loads and on bamboo reinforced concrete two-way slabs with unsupported boundaries subjected to non-central one loading. In the one-way slab, Kankam et al. (1986) reported that three different modes of failure were observed: (i) crushing of concrete in compression, which is symptomatic of no reinforcement, (ii) buckling failure as well as failure of concrete in compression and (iii) bamboo failure in tension. For the two-way slabs, they noted the yield line theory. Bolade (1990) reported that bamboo reinforcement considerably increased the load capacity of the beams at ultimate load up to an optimum value compared to an unreinforced member. The load increased to about 4–5 times more than that of plain concrete of equal dimension. Mignot (1977) indicated that the load capacity of bamboo reinforced concrete increased by using bamboo splines as diagonal tension reinforcement along the width of the beams where the vertical shear was generally high. There is therefore the need to define the limit to which bamboo can be incorporated in concrete to maximise its use in structural elements.

The objectives of these studies are to: (i) Assess the structural performance of bamboo under imposed uniformly distributed load, and (ii) Determine the extent to which

bamboo can be safely used as reinforcement in beams using different intensities of loading and spans.

2.6 EXPERIMENTAL PROCEDURE

The particles of the fine aggregate (sharp sand) used for the compressive strength test were those passing sieve with aperture 4.75mm and retained on sieve with aperture 0.075mm, the content is Ordinary Portland cement whose properties conform to British standard BS12 (1971). The mix proportion of 1:2.4 (cement : sand : granite chips) with water/cement ratio of 0.55 was used. 100s, 100s, 135mm concrete cubes were cast, cured and tested. The preparation and curing of the specimens were in accordance with BS1881 (1973). The strength of each cube was determined on a RIKEN Avery Denison Universal Testing machine using a loading rate of 1200N/min.

2.1 STRUCTURAL DESIGN

Generally, the design of structural elements is based on the ultimate and serviceability limit states.

2.1.1 Ultimate limit state

The ultimate limit state examines the strength of concrete beams.

(i) Bending

24 simply supported beams were designed over varying effective spans of 3.0, 3.5, 4.0 and 4.5 metres. The beams were subjected to 20%, 40%, 60%, 80% and 100% of the ultimate moment of resistance of the section. They have uniform cross sectional area of 225mm wide by 450mm deep. Fig. 1 shows the loading arrangement on each beam.

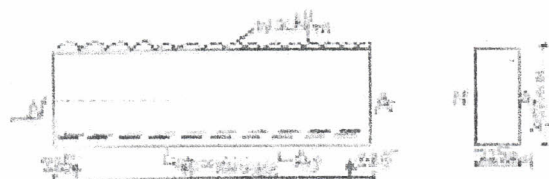


Fig. 1. Simply supported beam with imposed uniformly distributed load.

A_s = area of longitudinal tension reinforcement

The following assumptions were made during the design of the beams.

- The bamboo splints were obtained from natural veins.
- The splints were water protected with 2 coats of bituminous paint and therefore the durability of the splints was ensured.
- The concrete splints were coated with copper sand for effective bond with concrete matrix.

The moment of resistance of the beam was computed from equation

$$M_u = 0.156 b d^2 f_c \dots \dots \dots \text{BS 8110 (1985)}$$

where

M_u = Ultimate moment of resistance

b = Width of the beam

d = Effective depth

f_c = characteristic compressive strength of concrete at 28-day curing age

The lever arm factor was evaluated from equation

$$\frac{M_u}{M_u} = 0.89 (1 - \frac{M_u}{M_u}) \dots \dots \dots (1)$$

Mosley and Bungey (1983)

where

$\frac{M_u}{M_u}$ = lever arm factor

The area of reinforcement was calculated from equation

$$A_s = \frac{M_u}{0.54 f_y d} \dots \dots \dots (2)$$

Falade and Alejo (2002b).

where

M_u = Ultimate moment of resistance

A_s = Area of Bamboo splints

f_y = Stress in Bamboo splints

d = Effective depth

The appropriate quantity of bamboo splints whose area equals or higher than the area computed in the equation was selected.

(ii) Shear

The maximum shear force on the beam was computed by applying the equation for determining the value of the shear force on a beam subjected to uniformly distributed load:

$$V = \frac{wl}{2} \quad (5)$$

where:

V = Shear force

w = uniformly distributed load

L = effective span

The shear stress for each loading was determined from equation:

$$\tau = \frac{V}{bd} \quad (6)$$

where:

τ = shear stress, b = width of the beam

d = effective depth of the beam, $(d = D - C - \frac{\phi}{2})$

D = overall depth, C = Concrete cover, ϕ =

Diameter of bamboo splint

$\phi = \text{Diameter of bamboo splint}$

2.1.2 Serviceability limit state.

The deflection of each beam due to the applied load was computed using the equation of deflection for uniformly distributed load

$$\delta = \frac{5wL^4}{384EI} \quad (7)$$

where:

δ = Deflection of beam

w = Intensity of uniformly distributed load

L = Effective span of the beam

E = Modulus of Elasticity ($1.5833 \times 10^4 \text{ N/mm}^2$)

I = Moment of inertia ($1.7085 \times 10^9 \text{ mm}^4$)

w was obtained from

$$M_{max} = \frac{wl^2}{8} \quad (8)$$

$$w = \frac{8M_{max}}{l^2} \quad (9)$$

The values of deflection obtained for each loading was compared with allowable deflection stipulated in BS 8110 with some modifications that took cognizance of the characteristics of bamboo.

$$\delta = \frac{L}{350} \quad (10)$$

The basic ratios given are for a uniformly distributed load and for limiting deflection to $\frac{1}{250}$ (2000).

In the earlier study by Falade and Akeju (2002a), it reported that within the limit of proportionality the deflection of bamboo reinforced beams was three times that of a steel reinforced beams thus the above equation for limiting deflection was factored by 1/3 to obtain

$$\frac{L}{1050} \quad (11)$$

for the allowable deflection and the basic ratio of 750 for limiting deflection respectively. The latter was used as a deflection in this study.

3.0. RESULTS AND DISCUSSIONS

(i) Preliminary test results

The results of the laboratory tests showed that 28-day characteristic strength of concrete is 25 N/mm^2 while the average tensile strength of bamboo is 131.50 N/mm^2 for splint of $10 \times 10 \text{ mm}$ cross sectional area.

(ii) Behaviour of beam under uniformly distributed load

Table 1. Calculated values of intensity of loading.

Maximum Capacity (%)	Span (m), Udl (kN/m)			
	3.50	3.50	4.0	4.50
20	25.6	18.81	14.4	11.38
40	51.2	37.62	28.80	22.76
60	76.8	56.42	43.20	34.14
80	102.40	75.23	57.6	45.52
100	128.00	94.04	72	56.89

Table 1 shows that for a given value of moment capacity, with increase in span the allowable value of ω_{dl} , which the beam can safely take, increases. Furthermore, the relationship between the ω_{dl} and moment capacity of each beam follows a linear function. At 30% loading, the value of ω_{dl} , which can be supported by beams of spans 3.0m is 124% more than the value of ω_{dl} which can be supported by 4.5m span. Also, at 100% M_u loading, it was observed that the 3.0m span beam will take ω_{dl} of 174% more than M_u load of the 4.5m span beam. There is no indication that the load bearing capacity of the beams relative to their respective spans reflects a near perfect linear relationship.

(iii) Shear stress in beams

Figure 2 shows the values of the shear stress for different intensities of loading for beams. The values increased with increase in the intensity of loading. For example, at 30% loading, the value is 0.41N/mm² for 3.0m span. For the same span, the stress values are 0.82, 1.23, 1.64 and 2.06N/mm² for 40%, 60%, 80% and 100% loading respectively. The same trend was observed for the other spans. Shear stress decreased when the span increased under a constant load. For 3.0m span at 40% load the stress is 0.82N/mm². At the same loading arrangement, the stress values are 0.71, 0.61 and 0.55N/mm² for 3.5m, 4.0m and 4.5m span respectively.

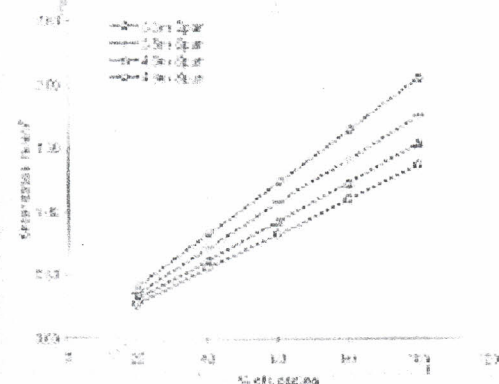


Fig. 2. Values of shear stress (N/mm²) versus percentage of loading

Generally, the stress values are lower than the ultimate value of 3.75N/mm² specified for 25N/mm² concrete.

(iv) Deflection of beams

Figure 3 indicates the values of deflection for each load case. The analysis carried out on the twenty four simply supported beams reinforced concrete beams that were subjected to uniformly distributed load showed that the beams deflected more under the same intensity of load when the span is increased.

The values of deflection are 1.0, 1.46, 1.77 and 2.24mm for the spans of 3.0, 3.5, 4.0 and 4.5m respectively. For a load of 25kNm (20% of M_u), whereas at 51.2kNm (40% of M_u) the results are 1.99, 2.71, 3.55 and 4.49mm for 3.0, 3.5, 4.0, and 4.5m respectively.

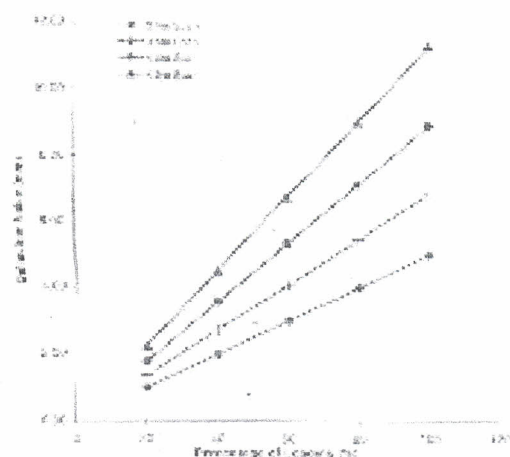


Fig. 3. Deflection (mm) versus Loading Case

The same trend was observed in other load cases (Figure 3). Thus, as an indication of the relative stability of the beams from the serviceability point of view. While the deflection-load curves of each beam shows linear relationship, it was observed that with increase in span and notwithstanding decrease in absolute value of ω_{dl} , the deflections of the beams

behave more pronounced. The allowable deflection values are 4.0mm for 3.0m span, 4.6mm for 3.5m while for 4.0 and 4.5m the values are 5.33mm and 6.0mm respectively.

Figure 4 presents the variation of deflections with different spans at different levels of loading. The figure shows that there exist a span for beams reinforced with bamboo that satisfies the requirements of both ultimate and serviceability limit state designs.

Figure 4 shows that for 3.0m span the beam could sustain 102.40kN/m (80% of the moment capacity). For 3.5m span, the permissible imposed load is 75.23 kN/m (50% of the moment capacity), while for 4.0m the allowable load is 43.20kN/m (60% of the moment of resistance of the section). In addition to these, for 4.5m the allowable load is 22.50kN/m about 50% of moment resistance of the section.

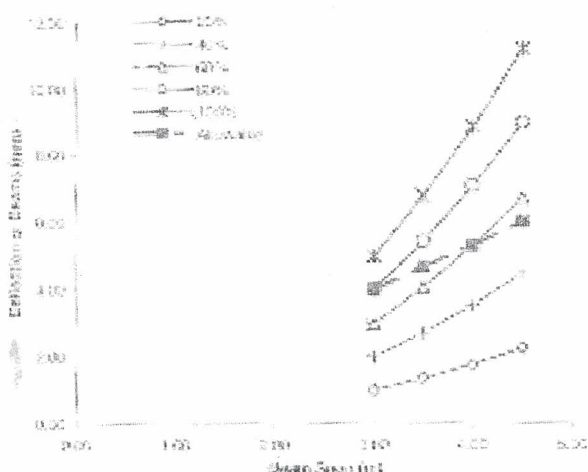


Fig. 4. Variation of Deflection with Span for different percentages of loading

It was observed that the value of deflection for 4.0m span was higher than the value for 3.0m span by 124% at a load level of 20% of moment of resistance (M_u). At 100% of M_u loading, for the 3.0m beam, deflection recorded was

4.95mm while for the 4.5m span beam deflection was 11.25mm, an indication of 127% increase in deflection.

4.0 CONCLUSIONS

From this study, the following conclusions are made:

1. Bamboo reinforcement can be used in concrete structures where the applied load is 60% of moment of resistance (M_u) of the beam span not exceeding 4.0m as both the ultimate and serviceability requirements are satisfied.
2. In bamboo-reinforced beams of same cross sectional areas, the allowable value of load the beams can support, decreased with increasing span.
3. In bamboo reinforced beams, value of deflection increased under the same intensity of load with span is increased.

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